

**INTERNATIONAL JOURNAL OF ENGINEERING SCIENCES & RESEARCH
TECHNOLOGY****PARAMETRIC STUDY OF SKEW ANGLE ON BOX GIRDER BRIDGE DECK****Shrikant D. Bobade * , Dr. Valsson Varghese**

* M-Tech Student, Structural Engineering, K D K College Of Engineering, Nagpur, India

Professor & Head, Department of Civil Engineering, K.D.K.College of Engineering, Nagpur, India

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ABSTRACT

Box girder bridge deck, is the most common type of bridges in world and India, it consists of several Slab or girders. The span in the direction of the roadway and connected across their tops and bottoms by a thin continuous structural slab, the longitudinal box girders can be made of steel or concrete. The Simple supported single span concrete bridge deck is presented in present study. Skewed bridges are suitable in highway design when the geometry of straight bridges is not possible. The skew angle can be defined as the angle between the normal to the centerline of the bridge and the centerline of the abutment or pier cap. Due to high traffic road can hardly modified in order to eliminate the skew. Therefore, considerable numbers of skew bridge decks are constructed. The skew angle effects on the behaviour of the bridge. Therefore, there is need for more research to study the effect of skew angle on performance of bridges. In the present study, the effect of change in skew angles with normal bridge is studied. Longitudinal moment, shear force, deflection and transverse moment are computed by modeling using STAAD-PRO with IRC loadings and results are compared.

KEYWORDS: Box girder bridge deck, skew angle, STAAD-PRO, IRC loadings.**INTRODUCTION**

Skew bridges are common at highway, river passage and other extreme grade changes when skewed geometry is necessary due to restrictions in space. There is a growing demand for skewed RC box girder bridges as the needs for complex intersection and the troubles with space constraint in urban and metro city areas arise. When roadway alignment changes are not feasible then the Skewed bridges are useful or due to the topography of the site to maintained economic and as well at particular areas someplace environmental impact is an issue. In order to offer high speeds and more safety necessities of the traffic, modern highways are to be straight as far as possible and this has required the provision of rising number of skew bridges. If a road alignment crosses a river or other obstruction at an inclination different from 90°, a skew crossing may be essential. The inclination of the centre line of Roadway to the centre line of river in case of a river bridge or other obstruction is called the skew angle. The analysis and design of a skew bridge are much more complicated than those for a Normal bridge. The analysis and design of bridge decks complicated if skew is present. Bridges with large angle of skew can have a considerable effect on the behaviour of the bridge especially in the various ranges of spans. A significant number of research studies have examined the performance of skewed highway bridges. However, there are no detailed guidelines addressing the performance of skewed highway bridges. Several parameters affect the response of skewed bridges which make their behaviour intricate. Therefore, there is a need for additional research to work the effect of skew angle on the performance of box girder bridges. Skew in a bridge can result from several factors, including natural or manmade obstacles, intricate intersections, space limitations, or mountainous terrain.

Characteristics of skew box girder bridge decks

In normal box girder bridges, the deck slab is perpendicular to the supports and the load placed on the deck slab is transferred to the supports which are placed normal to slab. Load transfer from a skew box girder slab bridge is complicated problem because there always remain a doubt as to the direction in which the slab and the manner in which the load will be transferred to the supports. With increasing the skew angle, the stresses in the box girder bridge deck and reactions on the abutment vary significantly from those in straight slab.

The magnitude and intensity of these effects depends on the angle of skew, aspect ratio of the slab and the type of construction of deck with supports. The shape and edge details can also influence the direction of maximum moments, the deck slab distance to abutments, the stiff edge beams acts as a line of support for the slab which effectively spans right to abutments crossways full width. The skew is so high that the deck is cantilevered off the abutments at the acute corners. The above characteristics are mostly significant in solid and cellular slab

decks because their high torsional stiffness try to oppose the twisting of deck. In contrast, the skew is less significant in beam and slab decks, particularly with spaced beams.

Effects of increase in the skew angle

With increasing the skew angle, the stresses in the slab differ significantly from straight slab. Loads applied on the slab are travels to the support in proportion to the rigidity of the various possible paths. Hence a major and important part of load tends to reach the support in a direction normal to the faces of piers and abutments. As a result, the planes of maximum stress are perpendicular to the centre line of the roadway and slab tends to twisted. The reactions at the obtuse angled corner of slab support are larger than other end, the increase in value common value ranging from 0 to 50% for skew angle of 15 to 55°. The reaction are negative for the skew angle more than 50°. The reaction on the obtuse angle end corner becomes twice the average reaction, thus creation the acute angle corner a zero pressure point when skew angle reaches about 60°.

OBJECTIVE AND SCOPE

- Analysis the behaviour of Box Girder Bridge for various skew angles by dynamic analysis.
- To compare normal and skew box girder bridge with parameter such as the Deflection, Support Reaction, Transverse Moment and Bending Moment by considering IRC Loading and Load Combination.

MODELLING AND ANALYSIS

- A Simply Supported Box Girder Bridge deck is considered.
- The bridge span considers are 40m and skew angle is varied from 0° to 60° at 5° intervals.
- The bridge is analyzed for the Dead load, live load i.e. IRC Loading are considered from the IRC 6-2014.
- Total 26 Numbers of Box Girder Bridge deck Models are generated and analyzed using STAAD-PRO software.

Table- 1 Material Properties

| Grade of Concrete | Density of Concrete | Poisson's Ratio | Elastic Modulus |
|--------------------------|----------------------------|------------------------|------------------------|
| M25 | 25kN/m ³ | 0.15 | 25Mpa |

LOADS ACTING ON BRIDGE

Dead and Superimposed Load

For general building structures, dead or permanent loading is the gravity loading due to the self weight structure and other items permanently attached to it. It is simply calculated as the product of volume and material density. Superimposed load is the gravity load of non-structural parts of the bridge. Such items are long term but may be changed during the life span of the structure. Thus, such superimposed dead loading is particularly prone to increases during the bridge life span. For this reason, a particularly high load factor is applied to road pavement. Bridges are extraordinary among structures in that a high proportion of the total loading is attributable to dead and superimposed dead load. This is mainly true of long-span bridges.

Live loads

Road bridge decks have to be designed to withstand the live loads specified by IRC (Indian Roads Congress i.e. I.R.C: 6-2014) in accordance with the specifications for the different and Various loads as well as stresses are consider in box girder bridge design. There are two types of typical loadings for which the bridges are designed specifically, IRC class A loading and IRC 70R.

a) For I.R.C. class A loading

The impact allowance is expressed as a fraction of the applied live load and is computed by the expression,

$$I=M/(N+L)$$

Where, I=impact factor fraction

M=constant having a value of 4.5 for a reinforced concrete bridges and 9.0 for steel bridges.

N=constant having a value of 6.0 for a reinforced concrete bridges and 13.5 for steel bridges.

L=span in meters. For span less than 3 meters,

The impact factor is 0.5 for a reinforced concrete bridges and 0.545 for steel bridges. When the span exceeds 45 meters, the impact factor is 0.088 for a reinforced concrete bridges and 0.154 for steel bridges.

b) For I.R.C. Class AA or 70R loading

(i) For span less than 9 meters

For tracked vehicle- 25% for a span up to 5m linearly reduced to a 10% for a span of 9m. For wheeled vehicles- 25%

(ii) For span of 9 m or more

- For tracked vehicle- for R.C. bridges, 10% up to a span of 40m.
- For steel bridges, 10% for all spans.
- For wheeled vehicles- for R.C. bridges, 25% up to a span of 12m.
- For steel bridges, 25% for span up to 23 meters.

COMBINATION OF LIVE LOAD

This clause shall be read in conjunction with Clause 112.1 of IRC: 5.The carriageway live load combination shall be considered for the design as shown in Table 2 of IRC 6:2014.

Table-II Live Load Combination

| Sl. No. | Carriageway Width (CW) | Number of Lanes for Design Purposes | Load Combination |
|---------|-------------------------------------|-------------------------------------|---|
| 1 | Less than 5.3 | 1 | One lane of Class A considered to occupy 2.3m. The remaining width of carriageway shall be loaded with 500 kg/m ² |
| 2 | 5.3m and above but less than 9.6m | 2 | One lane of Class 70R OR two lanes for Class A |
| 3 | 9.6m and above but less than 13.1 | 3 | One lane of Class 70R for every two lanes with one lanes of Class A on the remaining lane OR 3 lanes of Class A |
| 4 | 13.1m and above but less than 16.6m | 4 | One lane of Class 70R for every two lanes with one lane of Class A for the remaining lanes, if any, OR one lane of Class A for each lane. |
| 5 | 16.6m and above but less than 20.1 | 5 | |
| 6 | 20.1m and above but less than 23.6 | 6 | |

The live load combination are considered 3 lane for design are as mention below

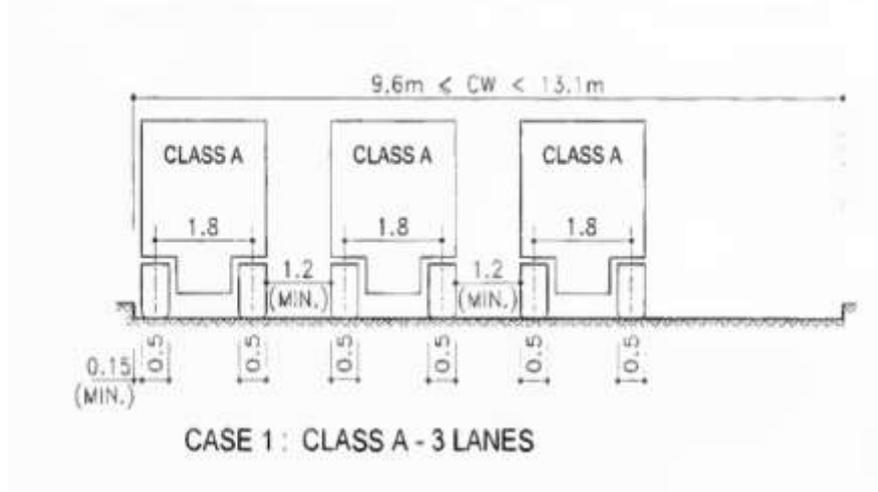


Fig. 3. IRC-6 : 2014 Live Load Combination Case I

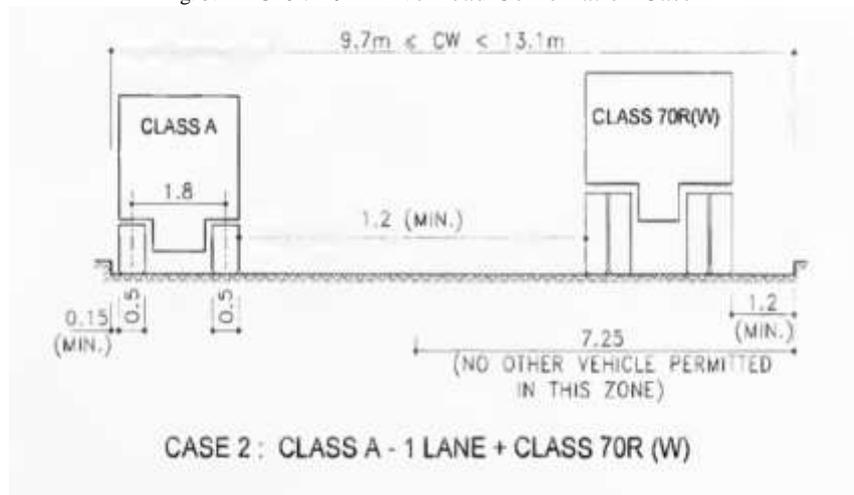


Fig. 4. IRC-6 : 2014 Live Load Combination Case II

DETAILS OF BOX GIRDER BRIDGE DECK

| | | | |
|-----------------------------|---|-------|-------------------|
| Length of Bridge | = | 40 | m |
| Width of Single Lane | = | 3.5 | m |
| Nos of Lanes | = | 3 | Nos |
| Clear Width of Roadways | = | 10.5 | m |
| Kerb Width | = | 0.6 | m |
| Width of Cantilever Bridge | = | 1.25 | m |
| Thickness of Top Slab | = | 0.25 | m |
| Thickness of Wearing Coat | = | 0.075 | m |
| Clear Width of Carriageway | = | 11.8 | m |
| Nos of Box Girders | = | 3 | Nos |
| Nos of Longitudinal Girders | = | 4 | Nos |
| Depth of Girder | = | 1.5 | m |
| Thickness of Girder | = | 0.3 | m |
| Thickness of Bottom Slab | = | 0.2 | m |
| Over All Length | = | 11.7 | m |
| Thickness of Kerb | = | 0.3 | m |
| Grade of Concrete | = | M | 25 |
| Grade of Concrete | = | Fe | 415 |
| Modular Ratio | = | 10 | |
| Density of Concrete | = | 25 | kN/m ³ |
| Density of Wearing Coat | = | 22 | kN/m ³ |

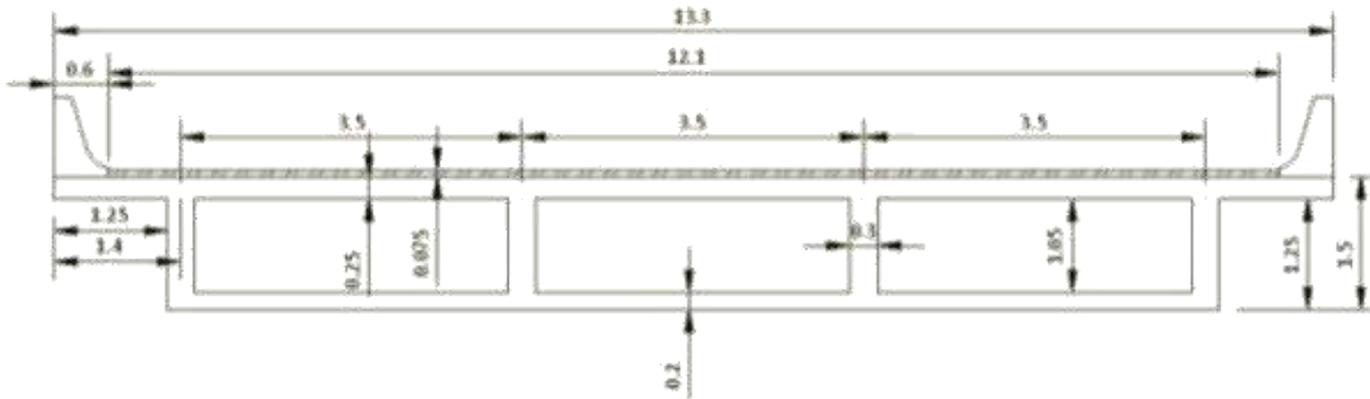
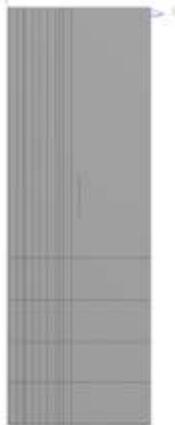


Fig.16 Bridge deck Model adopted for study

STAAD PRO MODELLING

Normal Skew Angle box Girder Bridge

60° Skew Angle Box Girder Bridge



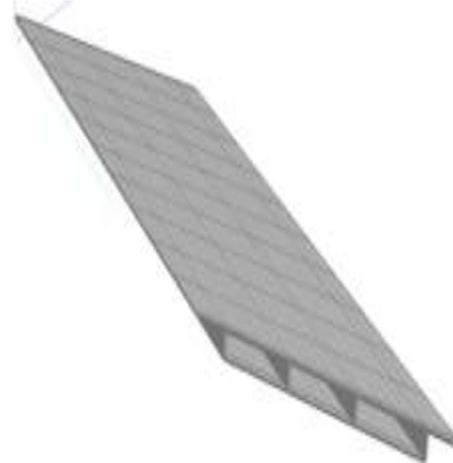
Plan



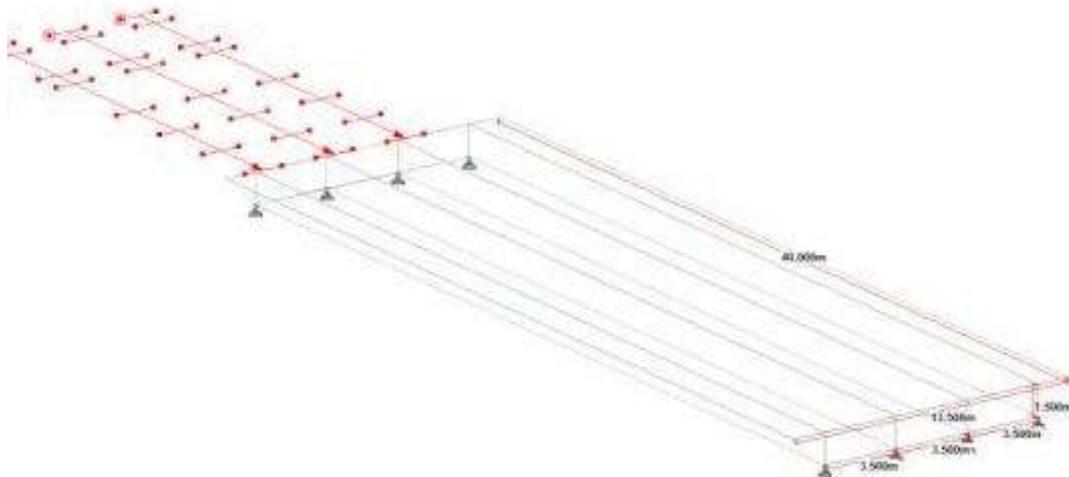
Plan



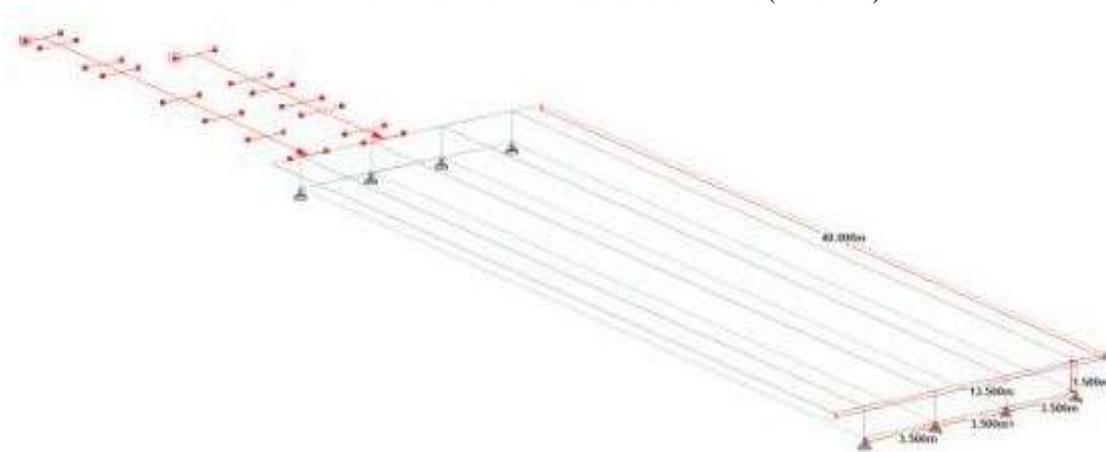
Isometric View



Isometric View



IRC-6 : 2014 Live Load Combination Case I (3 ClassA)



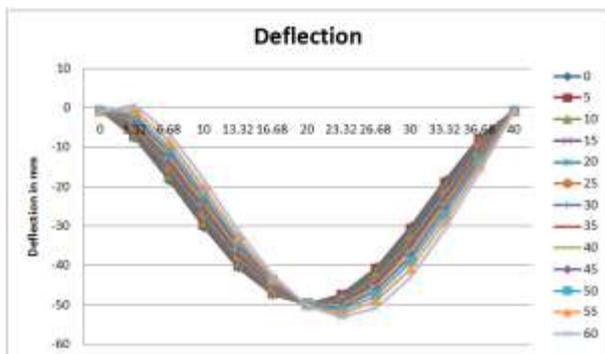
IRC-6 : 2014 Live Load Combination Case II (Class A + Class 70R(W))

RESULTS & DISCUSSIONS

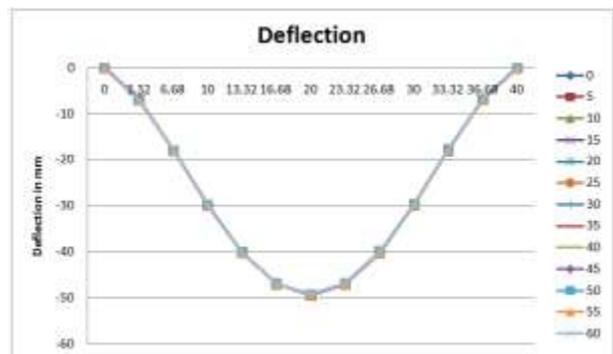
The results are obtained and the presented in terms of critical structural response such a deflection, longitudinal bending moment, torsional moment, shear force, and support reaction in the Box Girder Bridge deck Models due to the applied wheel load. The variations of the critical structural response parameter due to changes of skew angle are presented below.

Deflection

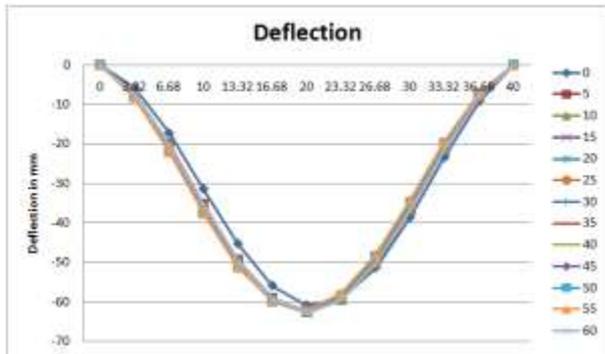
Dead Load (Outer girder)



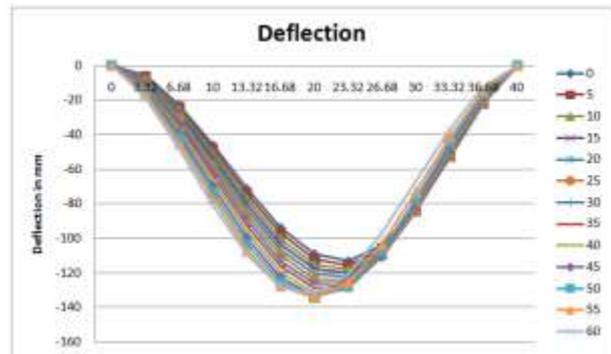
Dead Load (Inner girder)



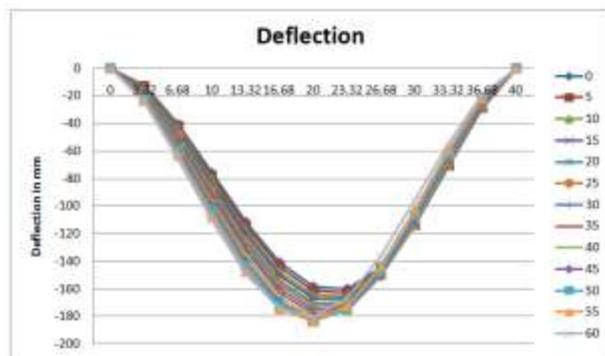
Live Load (Outer girder)



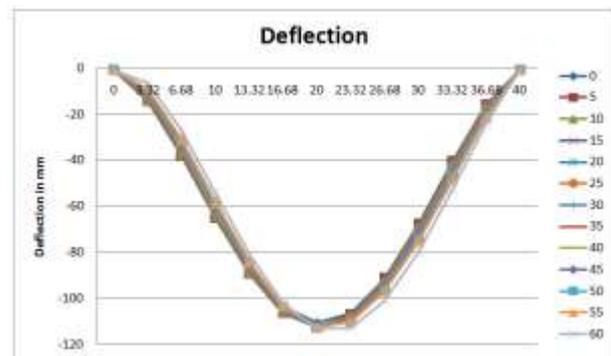
Live Load (Inner girder)



Total Load (Outer girder)

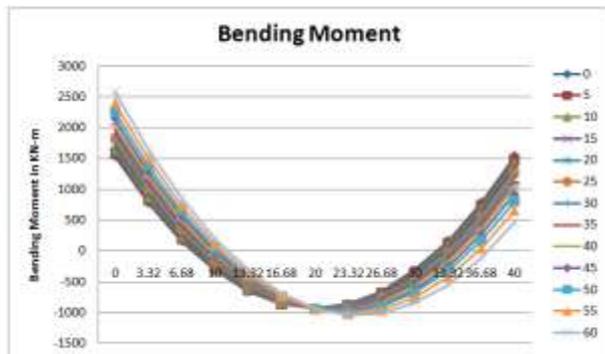


Total Load (Inner girder)

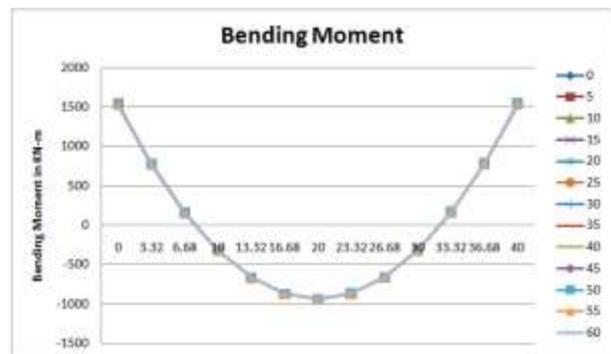


Bending Moment

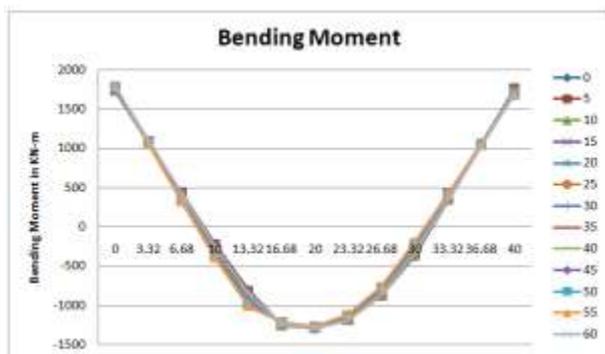
Dead Load (Outer girder)



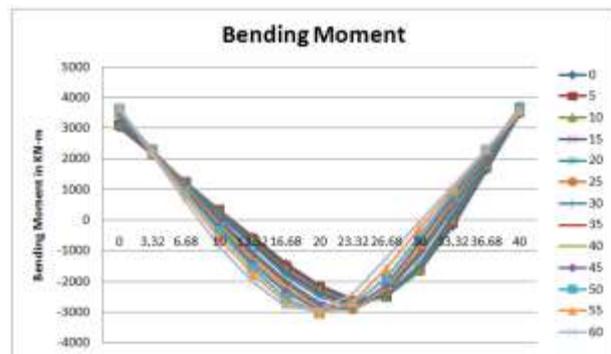
Dead Load (Inner girder)



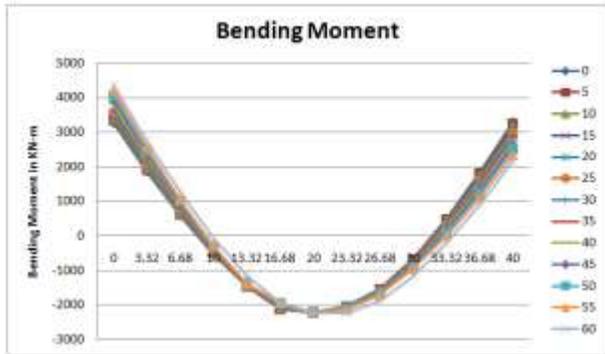
Live Load (Outer girder)



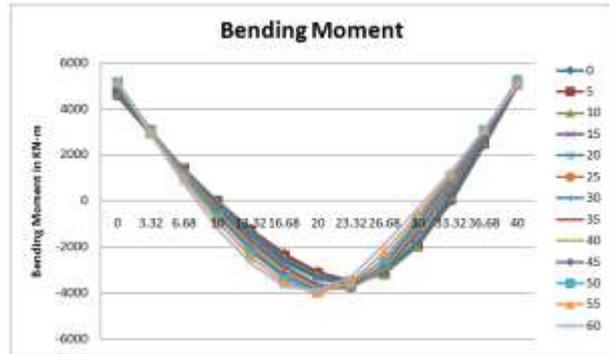
Live Load (Inner girder)



Total Load (Outer girder)

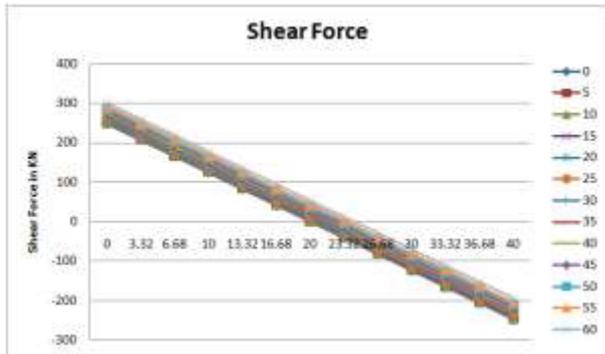


Total Load (Inner girder)

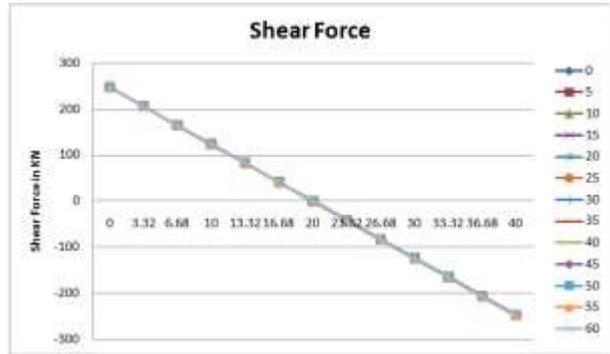


Shear Force

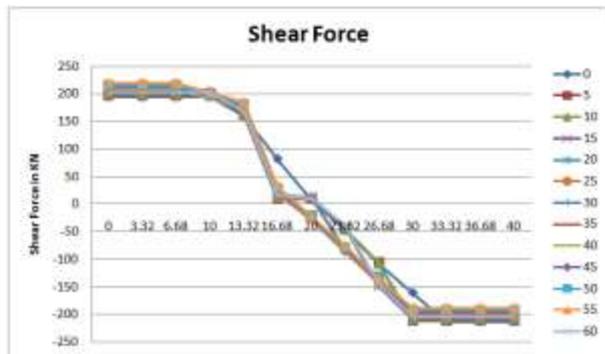
Dead Load (Outer girder)



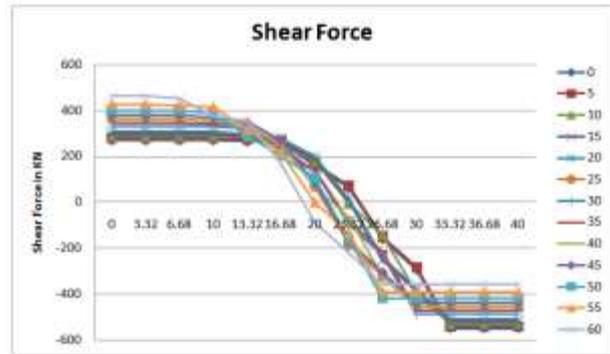
Dead Load (Inner girder)



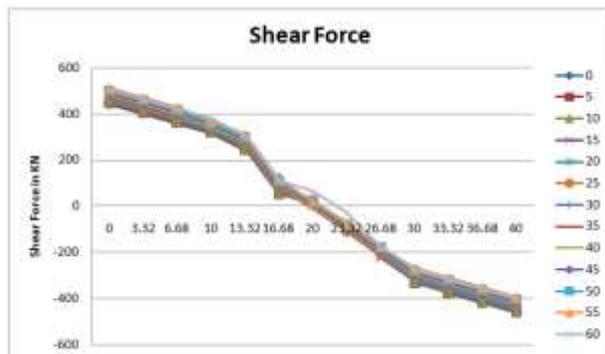
Live Load (Outer girder)



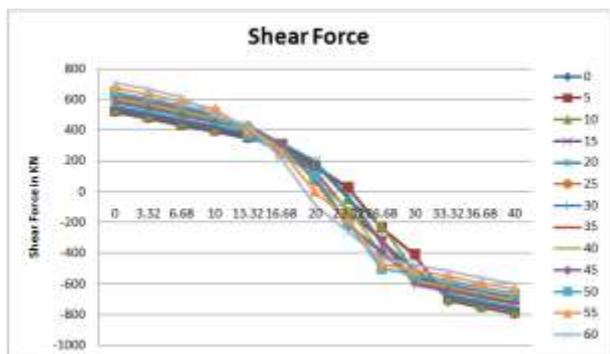
Live Load (Inner girder)



Total Load (Outer girder)

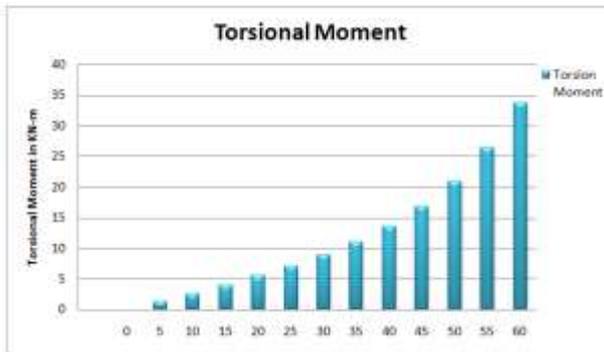


Total Load (Inner girder)

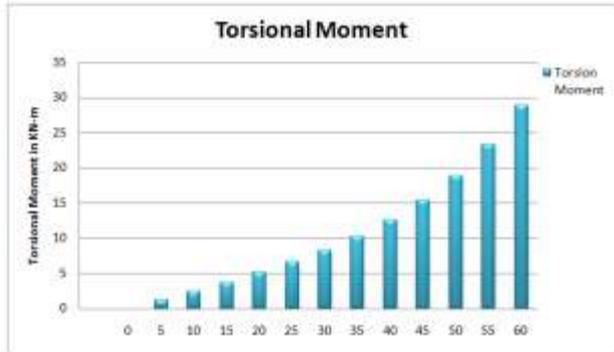


Torsional Moment

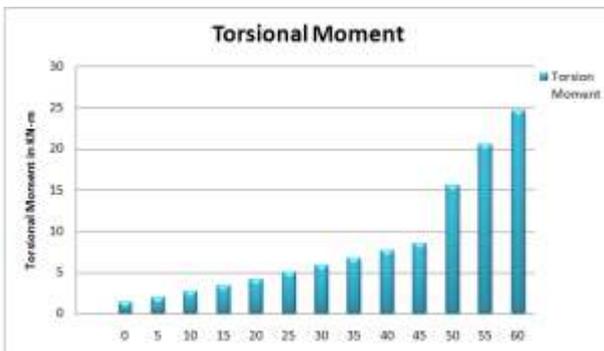
Dead Load (Outer girder)



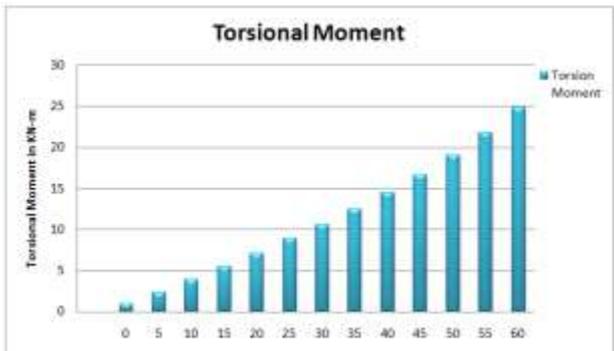
Dead Load (Inner girder)



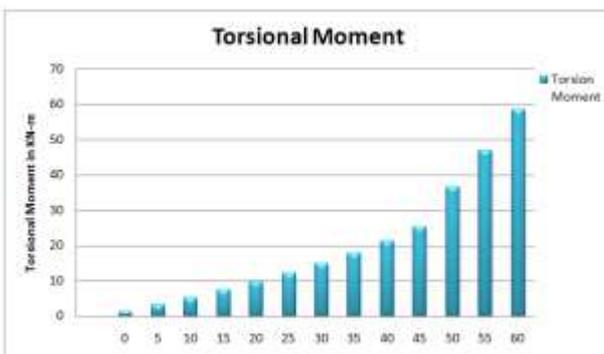
Live Load (Outer girder)



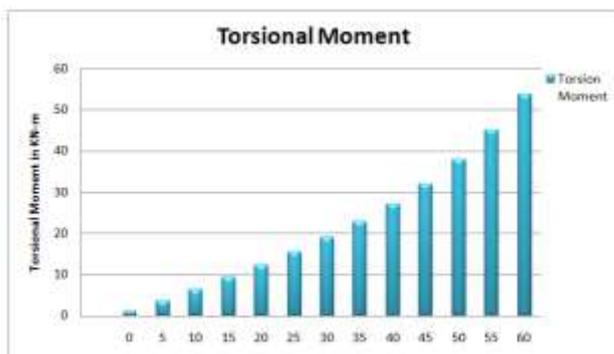
Live Load (Inner girder)



Total Load (Outer girder)



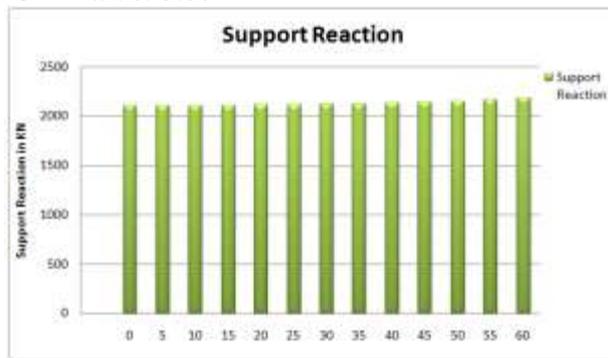
Total Load (Inner girder)



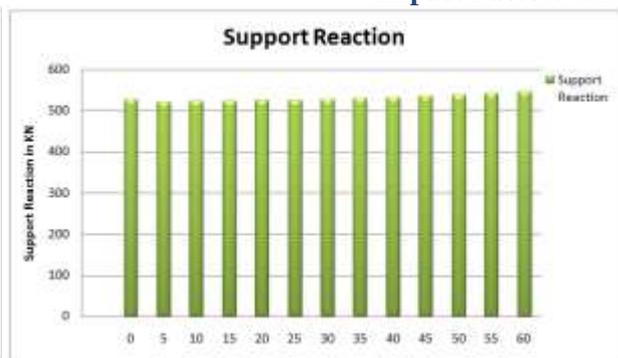
Support Reaction

Dead Load (Outer girder)

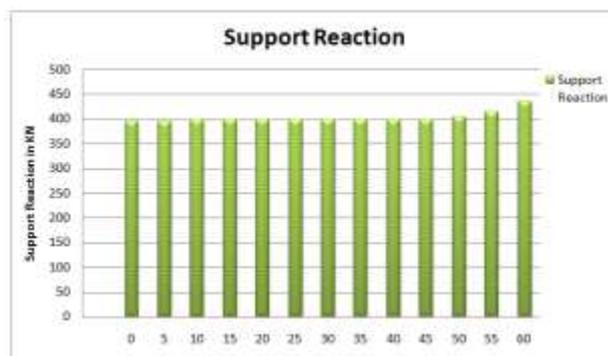
Dead Load (Inner girder)



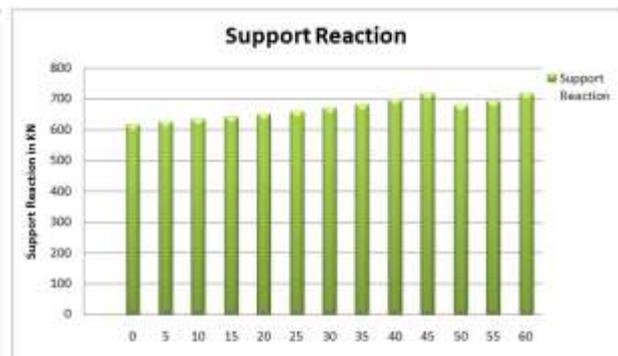
Live Load (Outer girder)



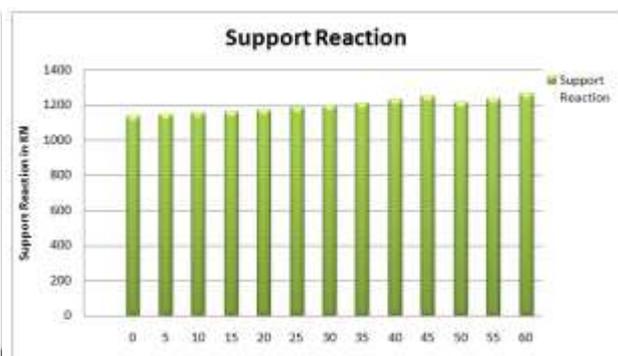
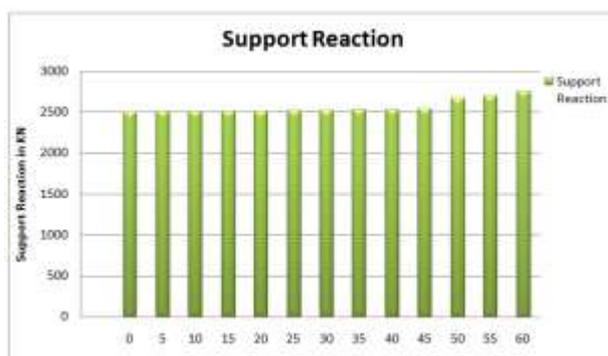
Live Load (Inner girder)



Total Load (Outer girder)



Total Load (Inner girder)



Parametric study is carried out on Three-lane box Girder Bridge for various Skew angles using Staad Pro and Deflection, Bending moment, Shear force, Torsional Moment and Support Reaction values were arrived at by Staad pro analysis for class A Tracked vehicle and Class 70R wheeled vehicle. These Staad Pro results are compared for various Skew angles.

- The results obtained for various Skew angles are presented in the form of graphs.
- Maximum Deflection occurs for Live Load Combination case - II. Hence Live Load Combination case - II is the most critical case for maximum Deflection in longitudinal girder are compared for various Skew angles.
- Maximum Bending moment occurs for Live Load Combination case - II. Hence Live Load Combination case - II is the most critical case for maximum Bending moment in longitudinal girder are compared for various Skew angles.
- Maximum Shear force occurs for Live Load Combination case - II. Hence Live Load Combination case - II is the most critical case for maximum Shear force in longitudinal girder are compared for various Skew angles.
- Maximum Torsional Moment occurs for Live Load Combination case - II. Hence Live Load Combination case - II is the most critical case for maximum Torsional Moment in longitudinal girder are compared for various Skew angles.

- Maximum Support Reaction occurs for Live Load Combination case - II. Hence Live Load Combination case - II is the most critical case for maximum Support Reaction in longitudinal girder are compared for various Skew angles.

CONCLUSIONS

The comparative study was conducted based on the analytical modeling of simply supported RC Box Girder Bridge for various Skew angles using Staad Pro. Based on this study Deflection occurs for Live Load Combination case - II of various Skew angles result is increase by (1.750%) with increase in Skew angle are compared. Bending moment occurs for Live Load Combination case - II of various Skew angles result is increase by (1.525%) with increase in Skew angle are compared. Shear force occurs for Live Load Combination case - II of various Skew angles result is increase by (1.376%) with increase in Skew angle are compared. Torsional Moment occurs for Live Load Combination case - II of various Skew angles result is increase by (135.36%) with increase in Skew angle are compared. Support Reaction occurs for Live Load Combination case - II of various Skew angles result is increase by (0.001%) with increase in Skew angle are compared.

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